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1. MASONRY CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE 2016 CALIFORNIA BUILDING CODE AND ACI 530/530.1, "BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES," LATEST

APPROVED EDITION. 2. CONCRETE MASONRY UNITS SHALL COMPLY WITH ASTM C 90, MEDIUM WEIGHT. THE COMPRESSIVE STRENGTH OF THE UNITS SHALL BE 1900psi.

REINFORCED MASONRY

3. THE COMPRESSIVE STRENGTH OF THE MASONRY ASSEMBLY, f'm = 1500psi.

4. ALL WALLS SHALL BE GROUTED SOLID. LIFTS SHALL NOT EXCEED 5'-0" IN HEIGHT UNLESS HIGHER LIFTS ARE APPROVED BY THE ENGINEER OF RECORD. CLEANOUT HOLES SHALL BE PROVIDED AT THE BASE OF ALL GROUTED CELLS, AND SHALL BE SPACED NO GREATER THAN 32" o.c. FOR SOLIDLY GROUTED MASONRY. GROUT SHALL BE MECHANICALLY VIBRATED.

5. GROUT SPACES SHALL BE FREE OF MORTAR DROPPINGS, DEBRIS, LOOSE AGGREGATES AND ANY OTHER DELETERIOUS MATERIALS.

6. GROUT CONSTRUCTION JOINTS SHALL BE FORMED BY STOPPING THE POUR 1 1/2" BELOW THE TOP OF THE UPPERMOST UNIT.

7. MORTAR SHALL CONFORM TO ASTM C270, TYPE S, WITH AN AVERAGE 28-DAY COMPRESSIVE STRENGTH OF 1800psi

8. GROUT SHALL CONFORM TO ASTM C476 AND SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2000psi.

9. REINFORCEMENT SHALL COMPLY WITH ASTM A615, GRADE 60. 10. PROVIDE VERTICAL BARS AT ENDS AND CORNERS OF MASONRY WALLS PER 5/S7.1 AT A MINIMUM, UNLESS

OTHERWISE NOTED ON PLANS. 11. CONTINUOUS REINFORCEMENT SHALL LAP PER 2/S7.1.

12. MASONRY CONSTRUCTION SHALL BE RUNNING BOND U.O.N.

13. REINFORCE OPENINGS PER 8/S7.1.

14. ALL JOINTS SHALL BE FULL BUTTERED.

GENERAL SPECIAL INSPECTION NOTES

1. SPECIAL INSPECTIONS, STRUCTURAL TESTS, AND STRUCTURAL OBSERVATIONS SHALL COMPLY WITH THE APPLICABLE PROVISIONS OF CHAPTER 17 OF THE 2016 C.B.C.

2. THE SPECIAL INSPECTOR MUST BE CERTIFIED BY THE CITY OF SAN DIEGO, DEVELOPMENT SERVICES, IN THE CATEGORY OF WORK REQUIRED TO HAVE SPECIAL INSPECTION.

3. THE SPECIAL INSPECTIONS IDENTIFIED ON PLANS ARE IN ADDITION TO AND NOT A SUBSTITUTE FOR THOSE INSPECTIONS REQUIRED TO BE PERFORMED BY A CITY BUILDING INSPECTOR.

4. A PROPERTY OWNER'S FINAL REPORT FORM FOR WORK REQUIRED TO HAVE SPECIAL INSPECTIONS, TESTING AND STRUCTURAL OBSERVATIONS MUST BE COMPLETED BY THE PROPERTY OWNER, PROPERTY OWNER'S AGENT OF RECORD, ARCHITECT OF RECORD OR ENGINEER OF RECORD AND SUBMITTED TO THE INSPECTION SERVICES

5. THE FABRICATOR MUST BE REGISTERED AND APPROVED BY THE CITY OF SAN DIEGO, DEVELOPMENT SERVICES FOR THE FABRICATION OF MEMBERS AND ASSEMBLIES ON THE PREMISES OF THE FABRICATOR'S SHOP

6. THE FABRICATOR SHALL SUBMIT A "APPLICATION TO PERFORM OFF-SITE FABRICATION" TO THE INSPECTION SERVICES DIVISION FOR APPROVAL PRIOR TO COMMENCEMENT OF FABRICATION.

7. THE FABRICATOR SHALL SUBMIT A "CERTIFICATE OF COMPLIANCE FOR OFF-SITE FABRICATION" TO THE INSPECTION SERVICES DIVISION PRIOR TO THE ERECTION OF FABRICATED ITEMS AND ASSEMBLIES.

8. THE CONSTRUCTION MATERIALS TESTING LABORATORY MUST BE APPROVED BY THE CITY OF SAN DIEGO. DEVELOPMENT SERVICES, FOR TESTING OF MATERIALS, SYSTEMS, COMPONENTS AND EQUIPMENT.

9. WHERE MATERIALS OR ASSEMBLIES ARE REQUIRED BY THE BUILDING CODE TO BE LABELED, SUCH MATERIALS AND ASSEMBLIES SHALL BE LABELED BY AN AGENCY APPROVED BY THE CITY OF SAN DIEGO, IN ACCORDANCE WITH C.B.C. SECTION 1703. PRODUCTS AND MATERIALS TO BE LABELED SHALL BE TESTED, INSPECTED AND LABELED IN ACCORDANCE WITH THE PROCEDURES SET FORTH IN C.B.C. SECTIONS 1703.5.1 THROUGH 1703.5.4. IDENTIFY ON PLANS THE NAME AND ADDRESS OF THE TESTING / INSPECTION AGENT.

ITEMS REQUIRING SPECIAL INSPECTION

. STRUCTURAL CONCRETE PER C.B.C. SECTION 1705.3 AND TABLE 1705.3.

2. MASONRY PER C.B.C. SECTION 1705.4 AND QUALITY ASSURANCE REQUIREMENTS OF TMS 602/ACI 530.1/ASCE 6. 3. EXISTING SITE SOIL CONDITIONS, FILL PLACEMENT AND LOAD-BEARING REQUIREMENTS PER C.B.C. SECTION 1705.6 AND TABLE 1705.6.

4. POST-INSTALLED EXPANSION ANCHORS IN CONCRETE. HILTI KB-TZ PER ICC ESR-1917

NOTICE TO THE APPLICANT/OWNER/OWNER'S AGENT/ARCHITECT OR ENGINEER OF RECORD

BY USING THIS PERMITTED CONSTRUCTION DRAWINGS FOR CONSTRUCTION/INSTALLATION OF THE WORK SPECIFIED HEREIN, YOU AGREE TO COMPLY WITH THE REQUIREMENTS OF THE CITY OF SAN DIEGO FOR SPECIAL INSPECTIONS, STRUCTURAL OBSERVATIONS, CONSTRUCTION MATERIAL TESTING AND OFF-SITE FABRICATION OF BUILDING COMPONENTS, CONTAINED IN THE STATEMENT OF SPECIAL INSPECTIONS AND, AS REQUIRED BY THE CALIFORNIA CONSTRUCTION CODES.

NOTICE TO THE CONTRACTOR/BUILDER/INSTALLER/SUB-CONTRACTOR/ **OWNER-BUILDER**

BY USING THIS PERMITTED CONSTRUCTION DRAWINGS FOR CONSTRUCTION/INSTALLATION OF THE WORK SPECIFIED HEREIN, YOU ACKNOWLEDGE AND ARE AWARE OF, THE REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTIONS. YOU AGREE TO COMPLY WITH THE REQUIREMENTS OF THE CITY OF SAN DIEGO FOR SPECIAL INSPECTIONS, STRUCTURAL OBSERVATIONS, CONSTRUCTION MATERIAL TESTING AND OFF-SITE FABRICATION OF BUILDING COMPONENTS, CONTAINED IN THE STATEMENT OF SPECIAL INSPECTIONS AND, AS REQUIRED BY THE CALIFORNIA CONSTRUCTION CODES.

DEFERRED SUBMITTALS

1. ROOF SAFETY TIEBACK DEVICES. CONTRACTOR TO PROVIDE DESIGN BUILD PACKAGE.

1. ALL FRAMING LUMBER SHALL BE DOUGLAS FIR-LARCH, GRADE MARKED AS FOLLOWS, UNLESS OTHERWISE NOTED: a. LIGHT FRAMING - "STANDARD" AND "STUD"

TIMBER

b. POSTS & BEAMS - "NO. 1"

c. BEARING STUDS & SHEAR WALL STUDS - "NO. 2" d. JOISTS, RAFTERS, PLATES - "NO. 2"

2. THE MOISTURE CONTENT IN USE SHALL NOT EXCEED 19 PERCENT.

3. SILL PLATES ON CONCRETE SHALL BE DOUGLAS FIR-LARCH, PRESSURE TREATED, WITH 5/8"dia HEADED ANCHOR BOLTS AT 4'-0"o.c. MAXIMUM U.O.N. (AT LEAST 2 ANCHOR BOLTS PER PIECE). PROVIDE 1/4"x3" SQUARE STEEL PLATE WASHER AND PROPERLY TIGHTENED NUT AT EACH ANCHOR BOLT. MINIMUM EMBEDMENT OF THE ANCHOR BOLTS INTO THE FOOTING SHALL BE 7" U.O.N.

4. SOLID BLOCK STUDS AT MAXIMUM 10'-0" VERTICAL INTERVALS.

5. INSTALL CONTINUOUS 2x SOLID BLOCKING BETWEEN ALL JOISTS AND RAFTERS AT SUPPORTS

6. CROSS BRIDGING BETWEEN JOISTS AND RAFTERS MAY BE OMITTED WHERE THE COMPRESSION EDGE IS CONTINUOUSLY CONNECTED TO THE SHEATHING. OTHERWISE, BRIDGE PER C.B.C. SECTION 2308.8.4.2.3. 7. ALL NAILS SHALL BE COMMON NAILS U.O.N.

8. SHEATHING NAILS SHALL BE DRIVEN SO THAT THEIR HEAD IS FLUSH WITH THE SURFACE OF THE SHEATHING.

9. BOLTS SHALL HAVE 7 DIAMETER MINIMUM END DISTANCE AND 4 DIAMETER MINIMUM EDGE DISTANCE. 10. ALL BOLTS SHALL BE FITTED WITH WASHERS. HOLES IN WOOD SHALL BE BORED WITH A BIT 1/32" TO 1/16" LARGER

THAN THE BOLT DIAMETER. 11. LEAD HOLES FOR LAG SCREWS AND WOOD SCREWS SHALL BE PROVIDED IN ACCORDANCE WITH THE "NATIONAL DESIGN STANDARD FOR WOOD CONSTRUCTION," LATEST APPROVED EDITION.

12. STEEL FRAMING CONNECTORS SHALL BE MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC. OR APPROVED

13. PLYWOOD FLOOR SHEATHING SHALL BE GLUED TO ALL FRAMING MEMBERS WITH AN APA APPROVED ADHESIVE. 14. ORIENTED STRAND BOARD (OSB) IS AN ACCEPTABLE ALTERNATE FOR PLYWOOD WALL, FLOOR, AND ROOF SHEATHING THAT IS NOT EXPOSED TO MOISTURE. OSB SHALL COMPLY WITH GRADE REQUIREMENTS NOTED ON

15. INSTALL FIRE BLOCKING IN CONCEALED SPACES OF STUD WALLS AND PARTITIONS, INCLUDING FURRED SPACES AT CEILING AND FLOOR SPACES AND AT OTHER LOCATIONS AS REQUIRED PER C.B.C. SECTION 717.

16. ALL HARDWARE, BOLTS, AND NAILS IN PRESERVATIVE-TREATED, FIRE-RETARDANT-TREATED OR EXPOSED LUMBER SHALL BE HOT-DIP GALVANIZED (ASTM A153) OR TYPE 304 STAINLESS STEEL UNLESS OTHERWISE RECOMMENDED BY THE MANUFACTURER.

GLUED LAMINATED BEAMS

 STRUCTURAL GLUED-LAMINATED BEAMS SHALL BE DOUGLAS FIR-LARCH, STRESS CLASS 24F-1.8E, AND SHALL BE MANUFACTURED IN ACCORDANCE WITH ANSI A190.1 (LATEST APPROVED EDITION). MINIMUM REFERENCE DESIGN VALUES SHALL BE AS FOLLOWS, U.O.N.:

a. POSITIVE FLEXURAL STRESS b. NEGATIVE FLEXURAL STRESS ... 1850 psi 265 psi c. HORIZONTAL SHEAR d. TENSION PARALLEL TO GRAIN. 1100 psi 1600 psi e. COMPRESSION PARALLEL TO GRAIN f. COMPRESSION PERPENDICULAR TO GRAIN 650 psi g. MODULUS OF ELASTICITY. 1,800,000 psi

2. GLUED-LAMINATED BEAMS THAT CANTILEVER OR ARE CONTINUOUS OVER SUPPORTS SHALL BE MANUFACTURED WITH A BALANCED LAYUP.

MOISTURE CONTENT SHALL BE LESS THAN 16 PERCENT, U.O.N.

4. THE REFERENCE DESIGN VALUES ABOVE SHALL BE MULTIPLIED BY ALL APPLICABLE ADJUSTMENT FACTORS. 5. CUTTING AND NOTCHING OF GLUED-LAMINATED BEAMS IS NOT PERMITTED UNLESS SPECIFICALLY DETAILED ON

THE STRUCTURAL PLANS. 6. LAMINATING ADHESIVES USED IN THE MANUFACTURE OF GLUED-LAMINATED TIMBER SHALL MEET THE REQUIREMENTS FOR THE SERVICE CONDITIONS SPECIFIED.

7. APPEARANCE GRADE SHALL BE INDUSTRIAL UNLESS OTHERWISE SPECIFIED BY THE ARCHITECT OF RECORD. 8. GLUED-LAMINATED MEMBERS SHALL BE PROVIDED WITH FACTORY-INSTALLED END SEALERS AND SHALL BE

INDIVIDUALLY WRAPPED PER THE RECOMMENDATIONS SET FORTH IN AITC 111 (LATEST APPROVED EDITION) U.O.N. 9. A CERTIFICATE OF CONFORMANCE INDICATING CONFORMANCE WITH ANSI A190.1 (LATEST APPROVED EDITION) SHALL BE PROVIDED TO THE BUILDING OFFICIAL PRIOR TO ERECTION.

LAMINATED VENEER LUMBER (LVL)

1. LVL USED FOR STRUCTURAL APPLICATIONS SUCH AS BEAMS, HEADERS, JOISTS, AND RAFTERS SHALL CONFORM TO THE FOLLOWING MINIMUM REFERENCE DESIGN VALUES:

a. FLEXURAL STRESS.. . 2900 psi b. HORIZONTAL SHEAR . 285 psi c. TENSION PARALLEL TO GRAIN.. 1660 psi d. COMPRESSION PARALLEL TO GRAIN. . 2635 psi e. COMPRESSION PERPENDICULAR TO GRAIN..... ... 750 psi f. MODULUS OF ELASTICITY.

. 2,000,000 psi

2. LVL USED FOR RIM JOISTS SHALL CONFORM TO THE FOLLOWING MINIMUM REFERENCE DESIGN VALUES: .2400 psi

a. FLEXURAL STRESS. b. HORIZONTAL SHEAR ...285 psi

c. TENSION PARALLEL TO GRAIN.. ..1450 psi d. COMPRESSION PARALLEL TO GRAIN... .. 2375 psi e. COMPRESSION PERPENDICULAR TO GRAIN..........750 psi f. MODULUS OF ELASTICITY. ..1,700,000 psi

3. LVL IS PERMITTED TO BE USED ONLY IN COVERED, DRY CONDITIONS. THE MOISTURE CONTENT OF LVL SHALL NOT EXCEED 16 PERCENT.

4. THE REFERENCE DESIGN VALUES ABOVE SHALL BE MULTIPLIED BY ALL APPLICABLE ADJUSTMENT FACTORS.

5. CUTTING AND NOTCHING OF LVL IS NOT PERMITTED UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL

6. THE USE OF TREATMENTS SUCH AS PRESERVATIVES OR FIRE-RETARDANTS ON LVL PRODUCTS IS NOT PERMITTED 7. THE CONTRACTOR SHALL SUBMIT ALL APPLICABLE MANUFACTURER PRODUCT DATA FOR REVIEW AND APPROVAL BY THE ENGINEER OF RECORD PRIOR TO PROCURING THE PRODUCT.

8. DESIGN CALCULATIONS, LAYOUT PLANS, AND DETAILS OF LVL SHALL BE SUBMITTED FOR REVIEW AND APPROVAL BY THE ENGINEER OF RECORD PRIOR TO PROCURING THE PRODUCT. THESE DOCUMENTS SHALL BEAR THE SEAL OF A CIVIL OR STRUCTURAL ENGINEER LICENSED IN THE STATE OF CALIFORNIA AND AUTHORIZED TO PERFORM

9. IF PRODUCTS ARE SUBMITTED THAT DO NOT COMPLY WITH THE MINIMUM REQUIREMENTS NOTED ABOVE, THE CONTRACTOR SHALL REIMBURSE THE ENGINEER OF RECORD FOR ALL TIME ASSOCIATED WITH REVISING THE STRUCTURAL PLANS AND RESUBMITTING TO THE APPROPRIATE JURISDICTION FOR APPROVAL.

1. FOR CHARACTER OF FOUNDATION SOIL, SEE SOILS REPORT BY LEIGHTON CONSULTING, INC, PROJECT NO. 11691.001, DATED JULY 28, 2017. OPTION 1: IMPORTED GRANULAR CAP.

2. THE ALLOWABLE SOIL BEARING PRESSURE IS 2000psf AT ISOLATED / COLUMN FOOTINGS AND 2000psf AT CONTINUOUS WALL FOOTINGS. THE ALLOWABLE SOIL BEARING PRESSURES MAY BE INCREASED BY ONE THIRD WHEN CONSIDERING LOADING OF SHORT DURATION SUCH AS WIND OR SEISMIC FORCES

FOUNDATION

3. FOOTINGS SHALL EXTEND TO A MINIMUM DEPTH OF 2'-0" BELOW LOWEST ADJACENT SOIL GRADE

4. PRIOR TO THE CONTRACTOR REQUESTING A BUILDING DEPARTMENT FOUNDATION INSPECTION, THE SOILS ENGINEER SHALL ADVISE THE BUILDING OFFICIAL IN WRITING THAT:

A. THE BUILDING PAD WAS PREPARED IN ACCORDANCE WITH THE SOILS REPORT

B. THE UTILITY TRENCHES HAVE BEEN PROPERLY BACKFILLED AND COMPACTED C. THE FOUNDATION EXCAVATIONS COMPLY WITH THE SOILS REPORT AND APPROVED PLAN.

5. REFER TO SOILS REPORT FOR RECOMMENDATIONS FOR REMOVAL OF COMPRESSIBLE SOILS, OVER EXCAVATION REQUIRED AND PRESOAKING OF SUBGRADE SOILS FOR BUILDING CONSTRUCTION

CONCRETE

CONCRETE SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH AS FOLLOWS:

A. FOUNDATIONS 4500psi. | 0.45 B. SLAB ON GRADE . 4500psi. | 0.45 C. WALLS AND ELEVATED SLABS ..

2. CEMENT SHALL COMPLY WITH ASTM C150, TYPE II OR V. CLASS F FLY ASH MAY BE SUBSTITUTED UP TO A MAXIMUM OF 15% BY WEIGHT OF CEMENTITIOUS MATERIALS IN NORMAL WEIGHT CONCRETE. SLABS ON GRADE SHALL HAVE 25% BY WEIGHT CLASS F FLY ASH.

3. NO PIPES OR DUCTS SHALL BE PLACED IN CONCRETE COLUMNS, WALLS, OR SLABS UNLESS SPECIFICALLY DETAILED ON STRUCTURAL DRAWINGS.

4. REINFORCEMENT SHALL BE SECURELY SUPPORTED IN POSITION PRIOR TO PLACING CONCRETE

5. WALLS AND COLUMNS SHALL BE DOWELED FROM SUPPORTS WITH BARS OF THE SAME SIZE AND SPACING U.O.N 6. SPLICE CONTINUOUS REINFORCEMENT WITH 2'-0" MINIMUM LAP U.O.N. PER 1/S7.1. BARS SPLICED WITH NON-CONTACT LAP SPLICES SHALL NOT BE SPACED TRANSVERSELY FARTHER APART THAN THE SMALLER OF ONE-FIFTH THE REQUIRED LAP SPLICE LENGTH AND 6". STAGGER SPLICES AT SLAB ON GRADE.

ALL REINFORCING STEEL SHALL BE DETAILED AND PLACED IN CONFORMANCE WITH THE "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI 318, LATEST APPROVED EDITION) AND THE "MANUAL OF STANDARD PRACTICE" BY CRSI, AS MODIFIED BY THE PROJECT DRAWINGS AND SPECIFICATIONS

8. REINFORCEMENT SHALL COMPLY WITH ASTM A615, GRADE 60, EXCEPT THAT REINFORCEMENT TO BE WELDED SHALL COMPLY WITH ASTM A706. REINFORCEMENT USED IN CONCRETE FRAME MEMBERS, SHEAR WALLS AND COUPLING BEAMS SHALL COMPLY WITH ASTM A706 UNLESS OTHERWISE APPROVED BY THE ENGINEER OF

9. ALL WELDING OF REINFORCING STEEL SHALL BE PERFORMED BY CERTIFIED WELDERS AND SHALL CONFORM TO AWS D1.4, "STRUCTURAL WELDING CODE -REINFORCING STEEL," LATEST APPROVED EDITION

STRUCTURAL STEEL

1. STRUCTURAL STEEL W-SHAPES SHALL COMPLY WITH ASTM A992 (Fy=50ksi). PLATES USED IN THE CONSTRUCTION OF MOMENT FRAMES AND BRACED FRAMES, INCLUDING BUT NOT LIMITED TO WEB DOUBLER PLATES, CONTINUITY PLATES, GUSSET PLATES, AND BASE PLATES, SHALL COMPLY WITH ASTM A572 GRADE 50 (Fy = 50ksi). ALL OTHER MISCELLANEOUS STRUCTURAL STEEL SHAPES AND BASE PLATES SHALL COMPLY WITH ASTM A36 (Fy=36ksi) U.O.N

MATERIALS AND WORKMANSHIP SHALL COMPLY WITH AISC 360. "SPECIFICATION FOR STRUCTURAL STEEL

BUILDINGS," LATEST APPROVED EDITION.

3. FABRICATION AND ERECTION OF STRUCTURAL STEEL MEMBERS AND CONNECTIONS IN MOMENT FRAMES AND BRACED FRAMES SHALL ALSO COMPLY WITH AISC 341, SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS,

LATEST APPROVED EDITION. 4. STEEL PIPE SHALL COMPLY WITH ASTM A53, GRADE "B" (Fy = 35ksi).

5. HSS SHALL COMPLY WITH ASTM A500, GRADE "B" (Fy=46ksi)

6. HIGH STRENGTH BOLTS SHALL COMPLY WITH ASTM A325N U.O.N.

7. MACHINE BOLTS SHALL COMPLY WITH ASTM A307 U.O.N.

8. HEADED ANCHOR BOLTS SHALL COMPLY WITH ASTM F1554 GRADE 36 U.O.N.

9. SHEAR STUDS SHALL BE FIELD WELDED AND MANUFACTURED BY NELSON STUD WELDING, INC. ICC ESR-2856 OR APPROVED EQUAL. 10. HIGH STRENGTH NON-SHRINK GROUT OR DRYPACK SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF

6,000psi U.O.N. 11. ALL WELDING SHALL BE BY THE SHIELDED ARC PROCESS. USE 70ksi LOW HYDROGEN ELECTRODES WITH

MATCHING FILLER METALS. 12. WELDING MAY BE PERFORMED IN THE SHOP OR IN THE FIELD AT THE DISCRETION OF THE CONTRACTOR. ALL

APPLICABLE SPECIAL INSPECTIONS AND TESTING SHALL APPLY BASED ON THE TYPE OF WELDING BEING PERFORMED. 13. ALL WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS AND SHALL CONFORM TO AWS D1.1, "STRUCTURAL

WELDING CODE-STEEL," LATEST APPROVED EDITION. WELDING OF JOINTS IN THE SEISMIC LOAD RESISTING SYSTEM SHALL ALSO CONFORM TO AWS D1.8 "STRUCTURAL WELDING CODE - SEISMIC SUPPLEMENT," LATEST APPROVED EDITION WHERE INDICATED ON PLANS. 14. WELDS INDICATED ON THE PLANS OR DETAILS TO BE DEMAND CRITICAL SHALL CONFORM TO THE APPLICABLE

REQUIREMENTS OF AWS D1.8. 15. ALL EXPOSED STRUCTURAL STEEL AND CONNECTORS SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM

A123 OR A153, AS APPLICABLE (LATEST APPROVED EDITION). 16. REPAIR AND TOUCH UP DAMAGED GALVANIZING ON STEEL PER ASTM A780 USING ZINC-RICH PAINT. 17. FURNISH TEMPLATES AND OTHER DEVICES AS NECESSARY FOR ACCURATE PRESETTING OF BOLTS. COORDINATE

BETWEEN TRADES AS REQUIRED. 18. CONTRACTOR SHALL BE RESPONSIBLE FOR BRACING OF STRUCTURAL STEEL WORK DURING ERECTION, WHICH SHALL BE ASSUMED TO BE NON-SELF SUPPORTING UNLESS THE CONTRACTOR INDEPENDENTLY VERIFIES

19. UNLESS OTHERWISE NOTED, STEEL BEAMS SHALL BE SPACED AT EQUAL SPANS BETWEEN COLUMN AND GIRDER LINES.

SPECIFICATIONS, THE INFORMATION SHOWN ON THE STRUCTURAL DRAWINGS

20psf

GENERAL

1. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS BEFORE STARTING WORK AND

2. DO NOT SCALE DETAILS OR DRAWINGS. SEE ARCHITECTURAL DRAWINGS FOR

3. SPECIFIC NOTES AND DETAILS SHALL TAKE PRECEDENCE OVER GENERAL NOTES

4. WHERE NO CONSTRUCTION DETAILS ARE SHOWN OR NOTED FOR ANY PART OF

5. WHEN A DETAIL IS IDENTIFIED AS TYPICAL, THE CONTRACTOR IS TO APPLY THIS

OR NOT THE REFERENCE IS REPEATED IN EVERY INSTANCE.

THE WORK, THE DETAILS USED SHALL BE THE SAME AS FOR OTHER SIMILAR WORK.

DETAIL IN ESTIMATION AND CONSTRUCTION TO EVERY LIKE CONDITION WHETHER

. ALL WORK SHALL CONFORM TO THE 2016 EDITION OF THE CALIFORNIA BUILDING

NOTIFY THE ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES THAT ARE FOUND.

1. APPLICABLE CODES: CALIFORNIA BUILDING CODE, 2016 EDITION

ADDITIONAL DIMENSIONS WHERE REQUIRED.

AND TYPICAL DETAILS.

OVER THE WORK.

EDITION, U.O.N.

SHALL GOVERN.

2. LIVE LOADS, UNIFORM AS FOLLOWS: A. ROOF. B. FLOORS.

4. SEISMIC DESIGN CRITERIA: SITE CLASS D

3. ROOF DEAD LOAD.

SEISMIC DESIGN CATEGORY D RISK CATEGORY II

 $F_a = 1.143$

 $F_{v} = 1.716$ $S_S = 0.893$

 $S_1 = 0.342$ $S_{DS} = 0.680$

 $S_{D1} = 0.391$ I = 1.0R = 5

 $V = C_sW = 39.056 lb$

 $C_S = S_{DS}/(R/I) = 0.136$

(EQUIVALENT LATERAL FORCE PROCEDURE)

5. WIND DESIGN CRITERIA:

 $V_U = 110 \text{ M.P.H.}$ WIND EXPOSURE C

RISK CATEGORY II $GC_{pi} = \pm 0.18$ $P = q_h (GC_p - GC_{pi})$

 $q_h = 22.4psf$

COMPONENTS AND CLADDING:

GC_D = SEE ASCE 7-10 CHAPTER 30, AS APPLICABLE

MAIN WIND FORCE RESISTING SYSTEM: C_p = SEE ASCE 7-10 CHAPTER 28, AS APPLICABLE

G = 0.85 $P = q(GC_p - GC_{pi})$

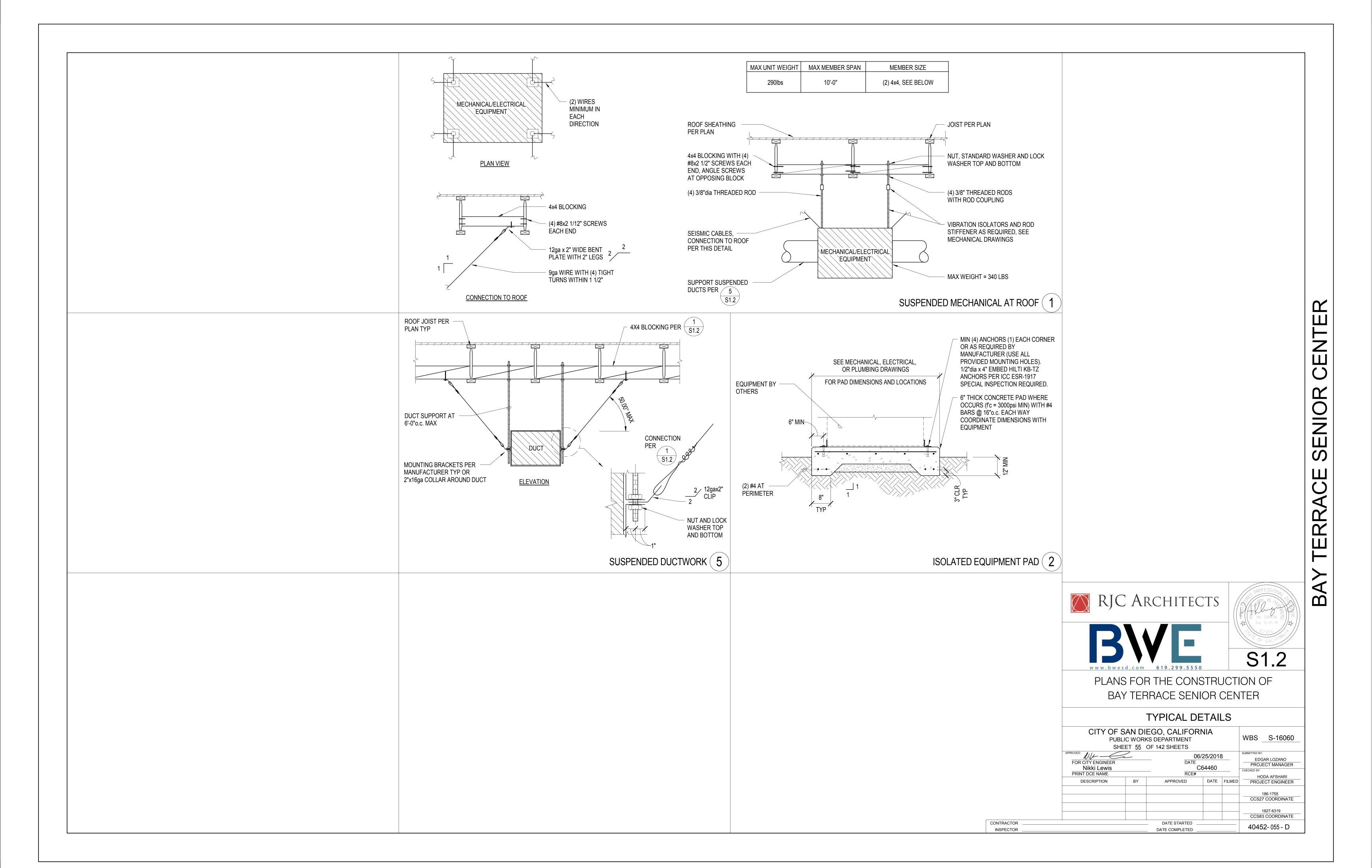


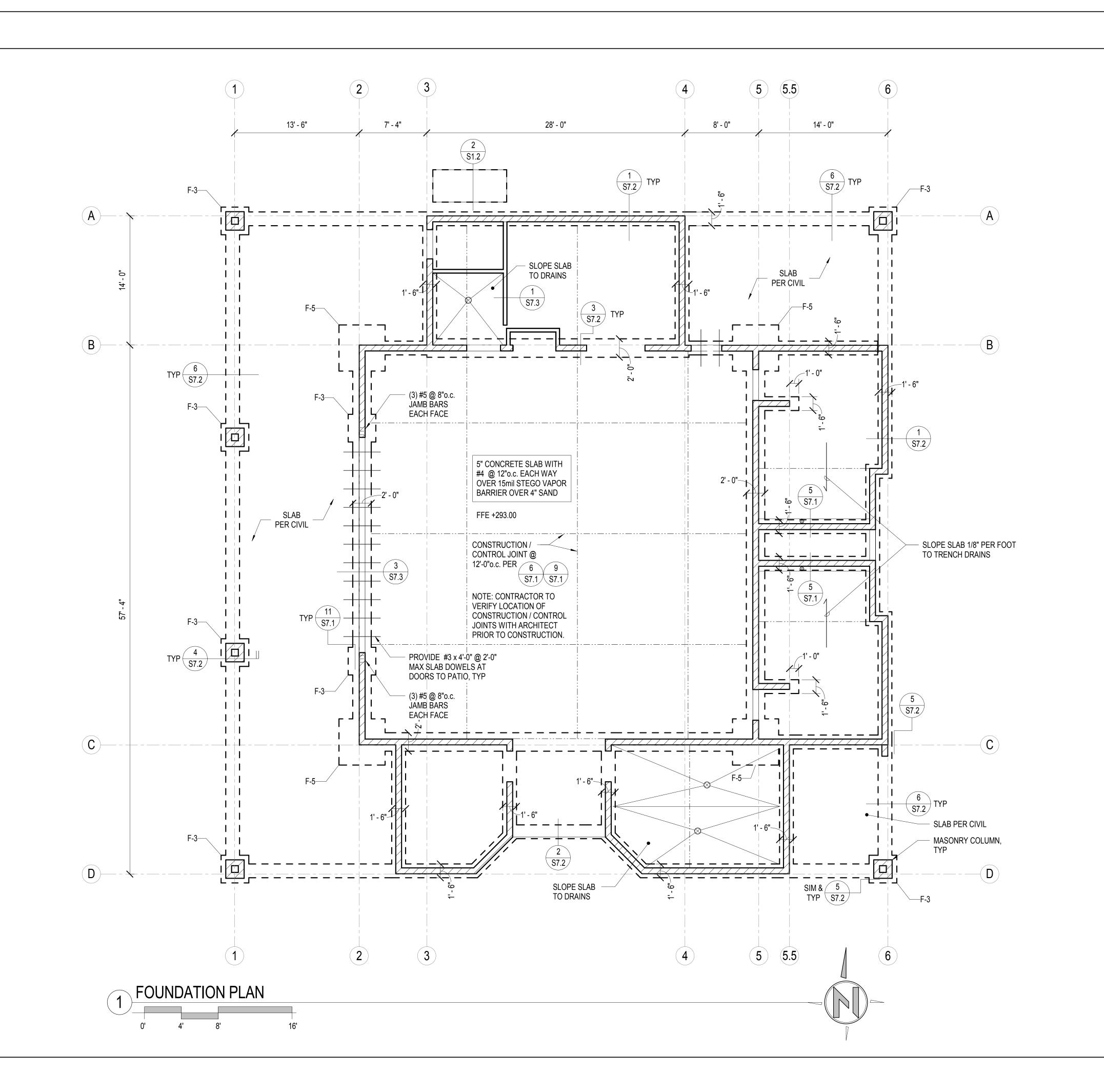
PLANS FOR THE CONSTRUCTION OF BAY TERRACE SENIOR CENTER

GENERAL NOTES

PUBLI	C WORK	EGO, CALIFORN S DEPARTMENT OF 142 SHEETS	AIV		WBS _	S-16060
FOR CITY ENGINEER Nikki Lewis	2	DATE	25/2018 64460	3		AR LOZANO ECT MANAGER
PRINT DCE NAME DESCRIPTION	BY	RCE#	DATE	FILMED		DA AFSHARI ECT ENGINEER
BEGGINI HEIV		781110125	21112	TIEMES	1	86-1755 COORDINATE
						827-6319 COORDINATE
		DATE STARTED			4045	2- 054 - D

CONTRACTOR INSPECTOR





NOTES

- 1. FOR ADDITIONAL NOTES AND INFORMATION, SEE S1.1.
- 2. SEE S7.1 AND S7.2 FOR TYPICAL DETAILS.
- 3. VERIFY ALL DIMENSIONS PRIOR TO START OF WORK, SEE ARCHITECTURAL
- DRAWINGS AS REQUIRED.
- 4. FOR LOCATION OF CURBS, SLAB DEPRESSIONS, SLAB SLOPES, FLOOR DRAINS, ETC., SEE THE ARCHITECTURAL DRAWINGS.
- 5. FOR CURBS, CONCRETE WALKS, FENCES, SITE WALLS, ETC. NOT SHOWN, SEE THE
- ARCHITECTURAL AND CIVIL DRAWINGS.

 6. FOR REINFORCEMENT AT THE INTERSECTIONS AND CORNERS OF GRADE BEAMS
- AND FOOTINGS, SEE DETAIL 8/S7.1
- 7. NON-BEARING/NON-SHEAR WALLS NOT SHOWN, SEE THE ARCHITECTURAL DRAWINGS.
- 8. SEE THE ARCHITECTURAL DRAWINGS FOR THE LOCATION AND SIZE OF ALL CONCRETE REVEALS, SCUPPERS, TEXTURES, ETC.
- 9. COORDINATE LOCAITON OF CMU CONTROL JOINTS WITH ARCHITECT. CONSTRUCT JOINTS PER 8/S7.3
- 10. PRIOR TO THE FOUNDATION INSPECTION THE CIVIL ENGINEER SHALL VERIFY THAT THE FOLLOWING ARE PER THE APPROVED GRADING PLANS.
- A. ALL FINISH PAD ELEVATIONS. B. ALL SETBACKS.

CONTRACTOR AT HIS/HER EXPENSE.

- 11. NOTE: THE RECOMMENDED EMBEDMENT FOR CONTINUOUS AND SPREAD
- FOOTINGS IS 24 INCHES BELOW TOP OF PAD.

 12. THE STRUCTURAL DESIGN IS BASED FOR EQUIPMENT WEIGHING 200lbs. MAX. IF
- THE CONTRACTOR CHOOSES OR IS ALLOWED TO SUBSTITUTE EQUIPMENT WEIGHING MORE THAN THE INDICATED LOAD ALL STRUCTURAL DESIGN REVISIONS AND ASSOCIATED APPROVALS SHALL BE BORNE AND EXECUTED BY THE

LEGEND

F.O.C.	DENOTES FACE OF CONCRETE
C.O.	DENOTES CONCRETE OPENING
	DENOTES STEP IN FLOOR SLAB OR ROOF SHEATHING
	DENOTES 8" MASONRY WALL
\otimes	DENOTES DRAIN PER PLUMBING DRAWINGS

FOOTING SCHEDULE

MARK		SIZE	REINFORCEMENT		
IVIARK	WIDTH	LENGTH	DEPTH	REINFORGEIVIENT	
F-3	3' - 0"	3' - 0"	1' - 6"	(3) #5 EACH WAY	
F-5	5' - 0"	5' - 0"	1' - 6"	(6) #5 EACH WAY	

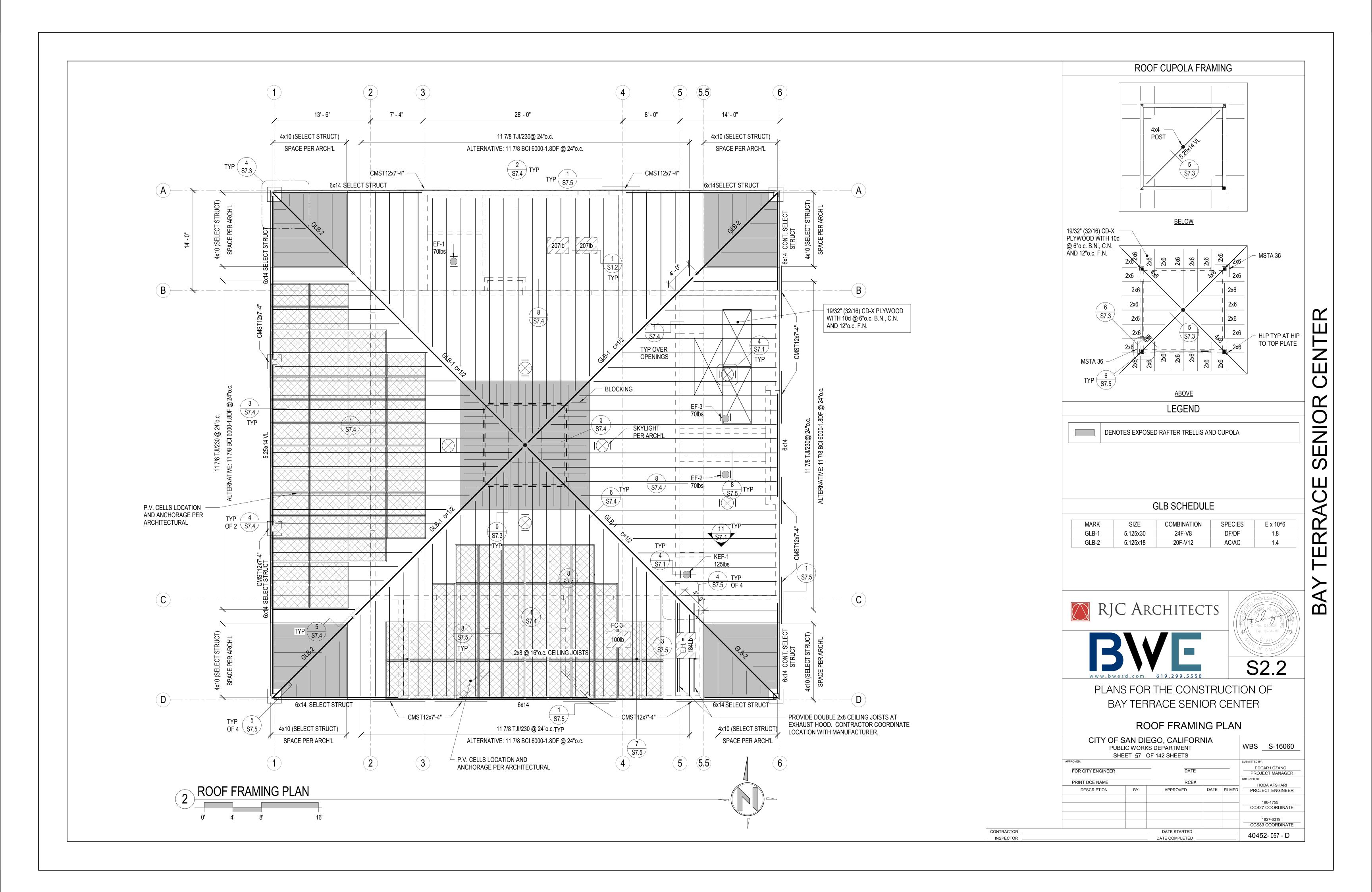


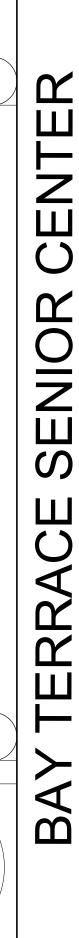
PLANS FOR THE CONSTRUCTION OF BAY TERRACE SENIOR CENTER

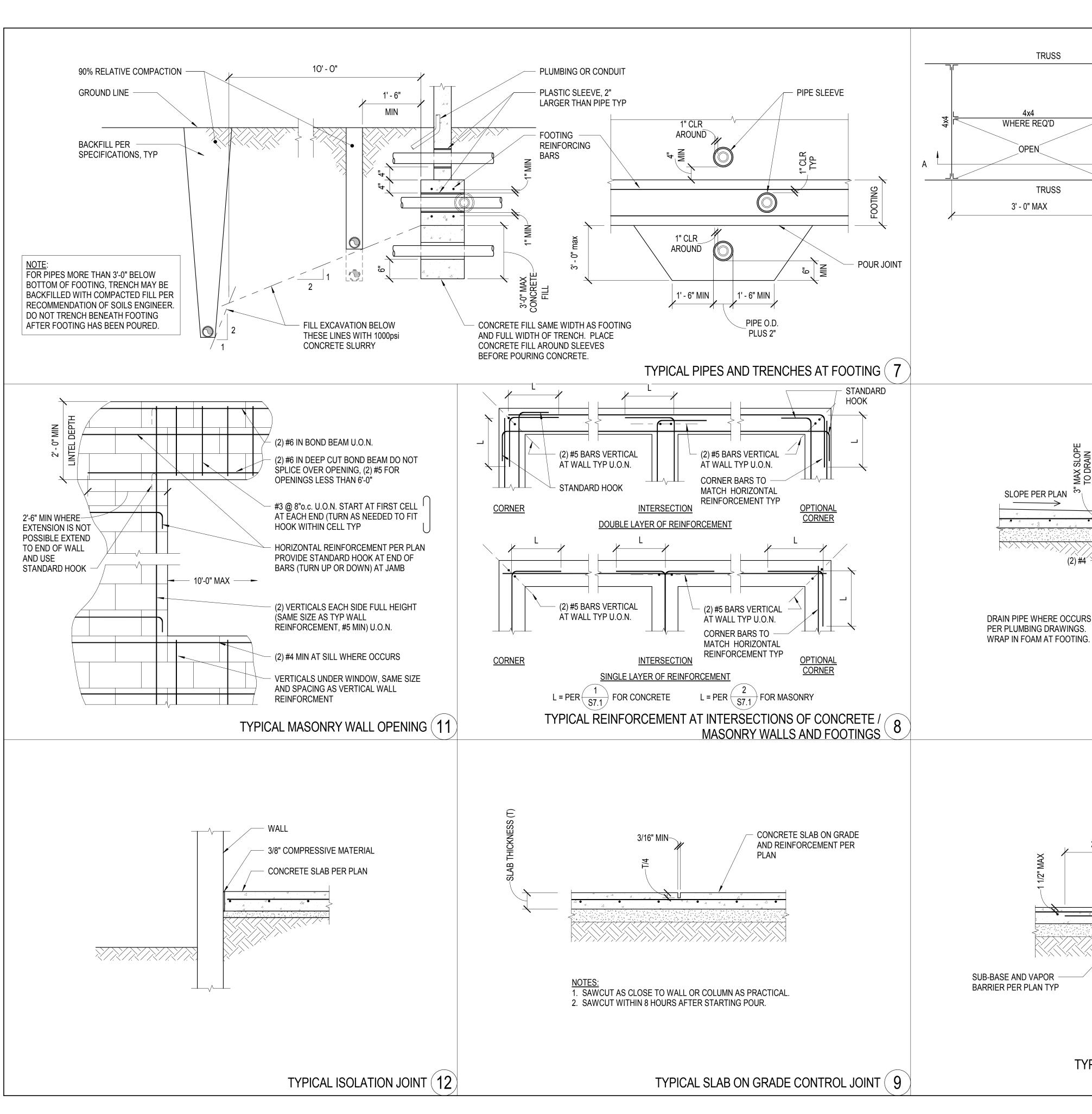
FOUNDATION PLAN

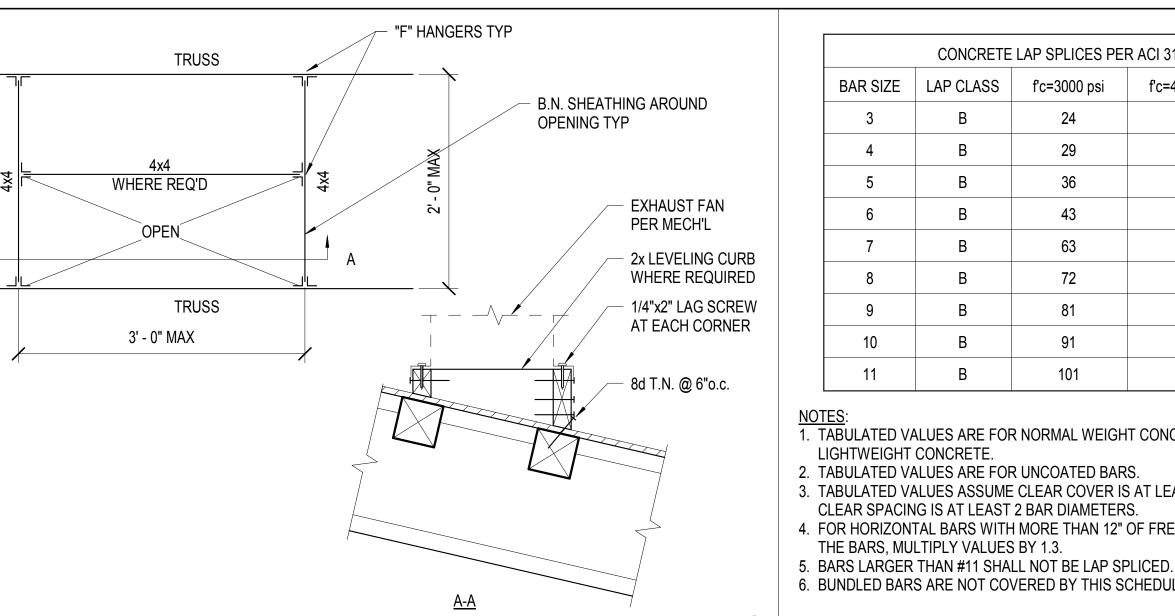
		EGO, CALIFORI S DEPARTMENT	AIV		WBS	S-16060
SHE	ET 56	OF 142 SHEETS				
FOR CITY ENGINEER Nikki Lewis	2	DATE	25/2018 64460	8		AR LOZANO ECT MANAGER
PRINT DCE NAME		RCE#	14400		CHECKED BY:	DA AFSHARI
DESCRIPTION	BY	APPROVED	DATE	FILMED	PROJE	CT ENGINEER
						86-1755
					CCS27	COORDINATE
					1	827-6319
					CCS83	COORDINATE
		DATE STARTED DATE COMPLETED			4045	2-056 - D

CONTRACTOR









TYPICAL ROOF OPENING (4

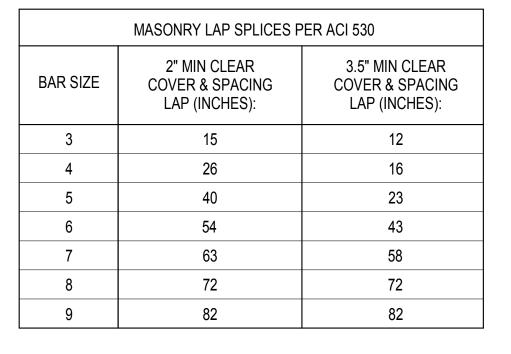
FOR ADDITIONAL NFORMATION, SEE S7.2

DRAIN TRENCH (5

	CONCRETE LAP SPLICES PER ACI 318 (INCHES)						
BAR SIZE	LAP CLASS	f'c=3000 psi	f'c=5000 psi				
3	В	24	24	24			
4	В	29	25	24			
5	В	36	31	28			
6	В	43	37	34			
7	В	63	54	49			
8	В	72	62	56			
9	В	81	70	63			
10	В	91	79	71			
11	В	101	87	78			

- 1. TABULATED VALUES ARE FOR NORMAL WEIGHT CONCRETE. DIVIDE BY 0.75 FOR
- 2. TABULATED VALUES ARE FOR UNCOATED BARS.
- 3. TABULATED VALUES ASSUME CLEAR COVER IS AT LEAST 1 BAR DIAMETER AND
- 4. FOR HORIZONTAL BARS WITH MORE THAN 12" OF FRESH CONCRETE CAST BELOW
- THE BARS, MULTIPLY VALUES BY 1.3.
- 6. BUNDLED BARS ARE NOT COVERED BY THIS SCHEDULE.

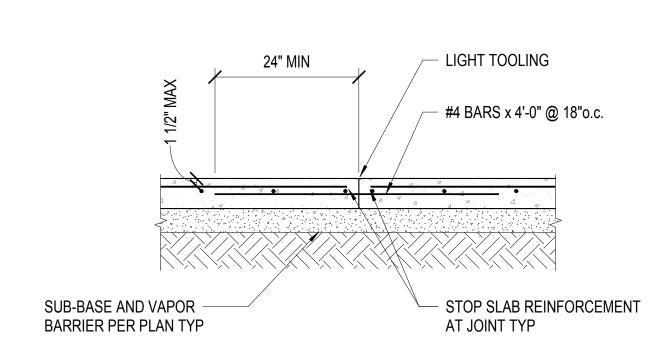
CONCRETE LAP SPLICE SCHEDULE (



- NOTES:

 1. TABULATED VALUES ARE FOR GRADE 60 REINFORCING BAR AND MINIMUM 1500 psi MASONRY.
- 2. TABULATED VALUES ARE FOR UNCOATED BARS. 3. TABULATED VALUES ASSUME MINIMUM CLEAR COVER AND SPACING AS
- 4. #9 BARS ARE THE LARGEST DIAMETER BARS ALLOWED FOR USE IN
- 5. BUNDLED BARS ARE NOT PERMITTED IN MASONRY.
- 6. FOLLOW ACI-530 FOR CIRCUMSTANCES NOT MEETING THESE ASSUMPTIONS.

MASONRY LAP SPLICE SCHEDULE (2



b



BAY TERRACE SENIOR CENTER

DETAILS

PUBL	IC WORK	EGO, CALIFORI S DEPARTMENT DF 142 SHEETS	NIA		WBS	S-16060	-
FOR CITY ENGINEER Nikki Lewis	2	DATE	25/2018 64460	3		AR LOZANO CCT MANAGER	-
PRINT DCE NAME DESCRIPTION	BY	RCE#	DATE	FILMED		DA AFSHARI CT ENGINEER	-
						86-1755 COORDINATE	-
						827-6319 COORDINATE	_
		DATE STARTED	•		4045	2 050 D	

DATE COMPLETED

40452-058 - D

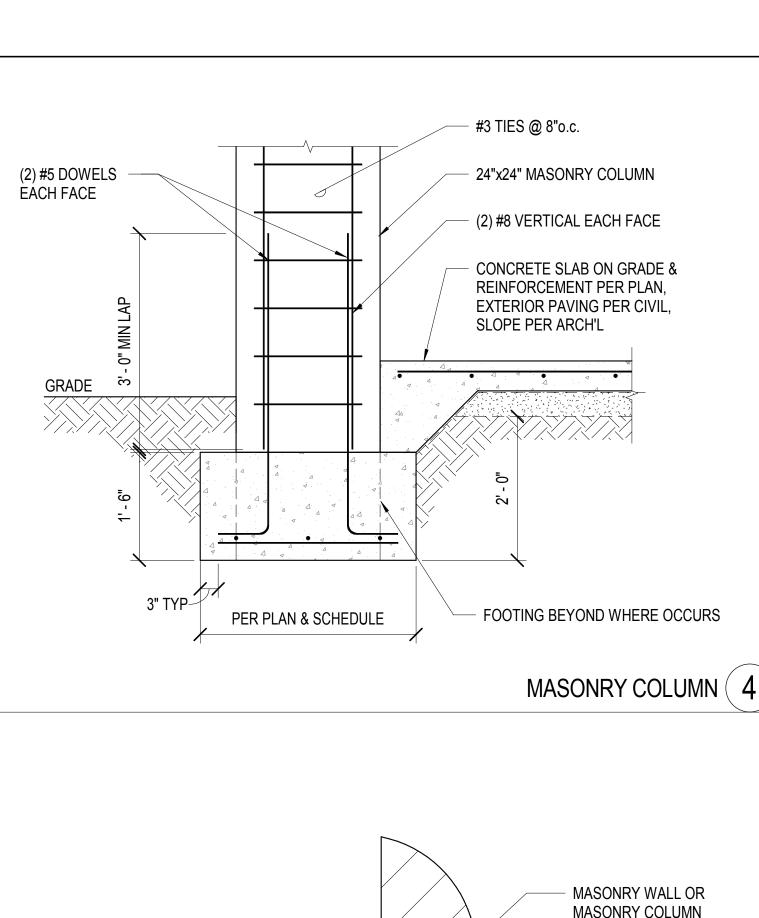
TYPICAL SLAB ON GRADE CONSTRUCTION JOINT (6

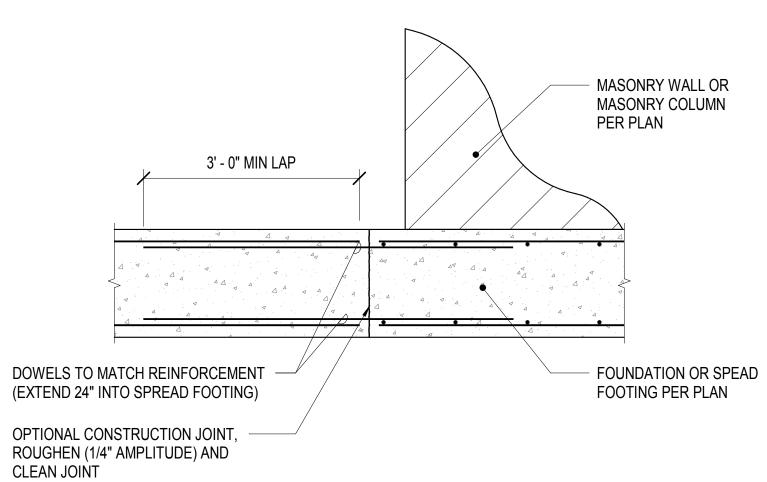
CONTRACTOR

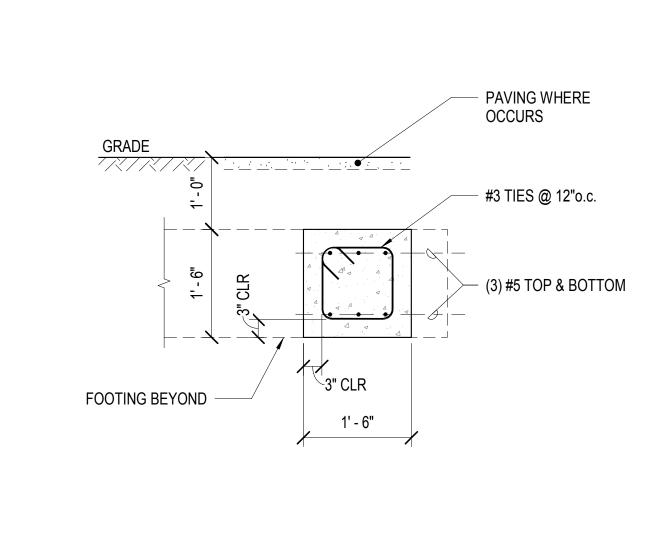
	NAILING SCHEDULE								
NO	CONNECTION	FASTENING	LOCATION						
1	JOIST TO SILL OR GIRDER	(3) 8d COMMON	TOE NAIL						
2	BRIDGING TO JOIST	(2) 8d COMMON	TOE NAIL EACH END						
3	1" x 6" SUBFLOOR OF LESS TO EACH JOIST	(2) 8d COMMON	FACE NAIL						
4	WIDER THAN 1" x 6" SUBFLOOR TO EACH JOIST	(3) 8d COMMON	FACE NAIL						
5	2" SUBFLOOR TO JOIST OR GIRDER	(2) 16d COMMON	BLIND AND FACE NAIL						
6	SOLE PLATE TO JOIST OR BLOCKING	16d @ 16"o.c.	TYPICAL FACE NAIL						
0	SOLE PLATE TO JOIST OF BLOCKING AT BRACED WALL PANEL	(3) 16" @ 16"o.c.	BRACED WALL PANELS						
7	TOP PLATE TO STUD	(2) 16d COMMON	END NAIL						
8	STUD TO SOLE PLATE	(4) 8d COMMON (2) 16d COMMON	TOE NAIL END NAIL						
9	DOUBLE STUDS	16d @ 24"o.c.	FACE NAIL						
40	DOUBLE TOP PLATES	16d @ 16"o.c.	TYPICAL FACE NAIL						
10	DOUBLE TOP PLATES EXCEPT AT SHEARWALLS	(8) 16d COMMON	LAP SPLICE						
11	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	(3) 8d COMMON	TOE NAIL						
12	RIM JOIST TO TOP PLATE	8d @ 6"o.c.	TOE NAIL						
13	TOP PLATES, LAPS AND INTERSECTIONS	(2) 16d COMMON	FACE NAIL						
14	CONTINUOUS HEADER, TWO PIECES	16d COMMON	16"o.c. ALONG EDGE						
	CEILING JOISTS TO PLATE	(3) 8d COMMON	TOE NAIL						
16	CONTINUOUS HEADER TO STUD	(4) 8d COMMON	TOE NAIL						
17	CEILING JOISTS, LAPS OVER PARTITIONS (SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1)	(3) 16d COMMON MIN, TABLE 2308.10.4.1	FACE NAIL						
18	CEILING JOIUSTS TO PARALLEL RAFTERS (SEE SECTION 2308.10.4.1, TABLE 308.10.4.1)	(3) 16d COMMON MIN, TABLE 2308.10.4.1	FACE NAIL						
19	RAFTERS TO PLATE (SEE SECTION 2308.10.4.1)	(3) 8d COMMON	TOE NAIL						
20	1" DIAGONAL BRACE TO EACH STUD AND PLATE	(2) 8d COMMON	FACE NAIL						
21	1"x8" SHEATHING TO EACH BEARING	(3) 8d COMMON	FACE NAIL						
22	WIDER THAN 1"x8" SHEATHING TO EACH BEARING	(3) 8d COMMON	FACE NAIL						
23	BUILT-UP CORNER STUDS	16d COMMON	@ 24"o.c.						
24	BUILT-UP GIRDER AND BEAMS	20d COMMON @ 32"o.c.	FACE NAIL AT T&B STAGGERED ON OPPOSITIONS SIDES						
		(2) 20d COMMON	FACE NAIL AT ENDS AND A EACH SPLICE						
25	2" PLANKS	16d COMMON	AT EACH BEARING						
26	COLLAR TIE TO RAFTER	(3) 10d COMMON	FACE NAIL						
27	JACK RAFTER TO HIP	(3) 10d COMMON (2) 16d COMMON	TOE NAIL FACE NAIL						
28	ROOF RAFTERS TO 2x RIDGE BEAM	(2) 16d COMMON	TOE NAIL						
		(2) 16d COMMON	FACE NAIL						
29	JOISTS TO BAND JOIST	(3) 16d COMMON	FACE NAIL						
30	LEDGER STRIP	(3) 16d COMMON	FACE NAIL AT EACH JOIS						
	WOOD STRUCTURAL PANELS AND PARTICLE BOARD SUBFLOOR, ROOF AND WALL	1/2" AND LESS	6d						
31	SHEATHING (TO FRAMING)	19/32" TO 3/4"	8d OR 6d						
JΙ		7/8" TO 1" 3/4" AND LESS	8d 6d						
	SINGLE FLOOR	7/8" TO 1"	8d						
		1 1/8" TO 1 1/4"	10d OR 8d						

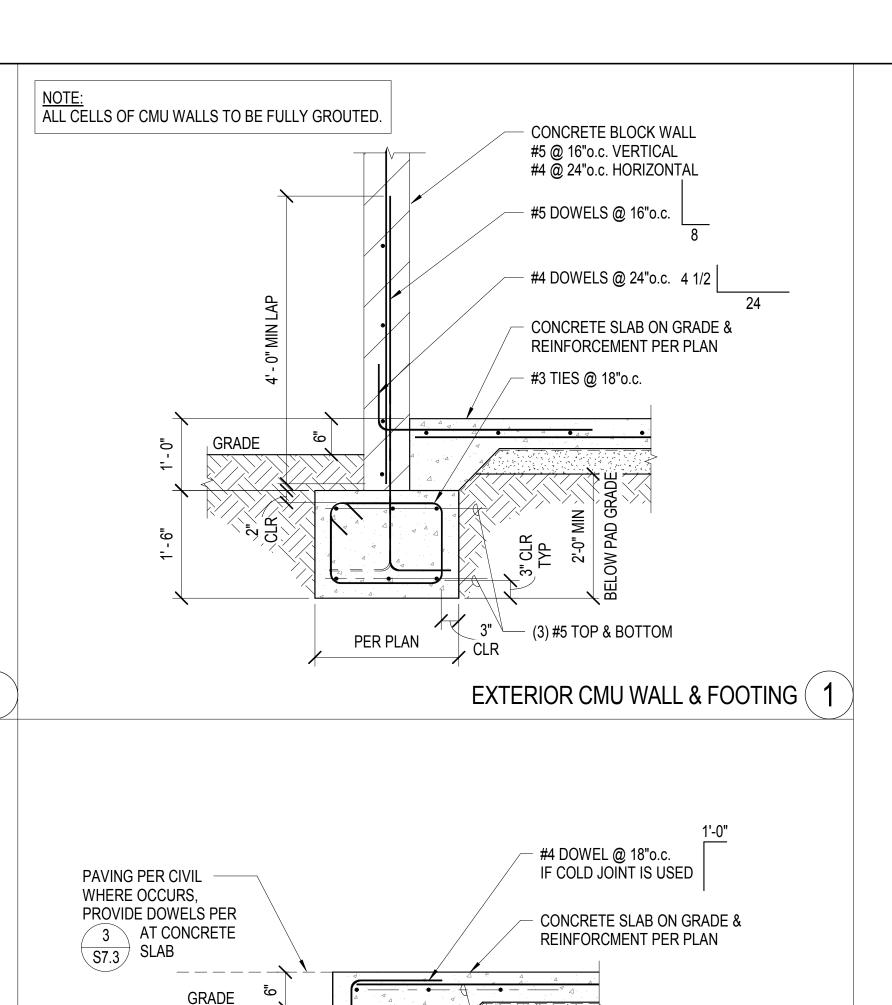
1. THE NUMBER AND SIZE OF FASTENERS SHOWN ABOVE ARE MINIMUM REQUIREMENTS. IF OTHER NOTES AND/OR DETAILS ON THE DRAWINGS REQUIRE MORE AND/OR LARGER FASTENERS, THE OTHER NOTES AND/OR DETAILS SHALL GOVERN.

- 2. WHEN POSSIBLE, NAILS DRIVEN PERPENDICULAR TO GRAIN SHALL BE USED INSTEAD OF TOENAILS.
- 3. SEE TABLE 2304.9.1 OF THE 2016 C.B.C. FOR ADDITIONAL NOTES, REQUIREMENTS OR OPTIONS.

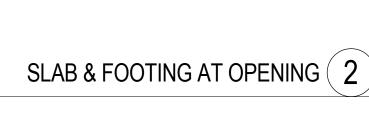




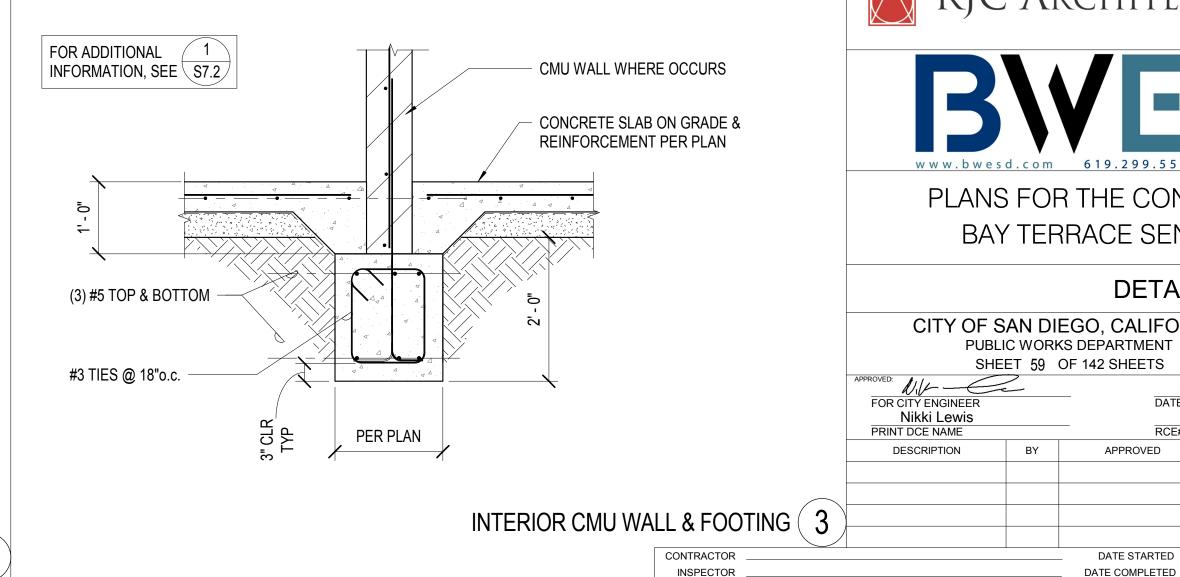




OPTIONAL COLD JOINT ROUGHEN & CLEAN



(2) #5 CONTINUOUS





EDGAR LOZANO PROJECT MANAGER C64460 HODA AFSHARI APPROVED PROJECT ENGINEER CCS27 COORDINATE 1827-6319 CCS83 COORDINATE DATE STARTED 40452-059 - D

TYPICAL NAILING SCHEDULE (9)

TIE BEAM (6)

FOOTING SPLICE (5)

